



Real-time updating procedure for flood forecasting in small flash flood exposed catchments

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SUMMARY

An increasing climatic variability leads to an increasing occurrence probability of extreme events. Especially in small flash flood exposed catchments with short reaction times accurate flood forecasting is highly important. Unfortunately the available rainfall-runoff models do, especially for extreme events in small catchments, not allow for an acceptable prediction accuracy. In order to use these models for flood forecasting for emerging flood events and to ensure an acceptable accuracy the model outputs have to be continuously updated using real time river gauge measurements. Various updating procedures have been published which are, however, mostly not suitable for short forecast periods and a steep flood hydrograph characteristic which is typical for small, quick reacting mountainous catchments. In such catchments it is the primary goal to extend the forecast lead time. This requires procedures that update the state variables that govern the runoff generation process of the used rainfall runoff model. Classical updating procedures (e.g. Auto Regression Moving Average approaches) are focussed on the river flow itself which leads to a significant loss of forecast lead time in small, quick reacting catchments. Therefore, it was our intention to develop a simple updating procedure that allows for the updating of sensitive state variables that control the runoff generation approach of an appropriate rainfall runoff model. At first we investigated WASIM-ETH model in this respect. The analysis yielded that this model was too complex for the envisaged purpose. As an alternative we investigated the model PREVAH (Precipitation-Runoff-Evapotranspiration-Hydrotope Model, Gurtz et al. 1999). This more conceptual rainfall-runoff model represents the catchment characteristics using hydrologic response units (HRU). Runoff generation processes are represented by linear reservoirs in this model. This allowed for the adjustment of the pre-event state of the system, i.e. the soil moisture content using a simple procedure that is based on the comparison between the predicted and the measured development of the flood wave. Several tests of the updating procedure using data from mountainous catchments in Switzerland and Germany showed a significant increase in prediction accuracy with respect to peak discharge at a very early state of the flood.

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1. Introduction

An increasing climatic variability leads to an increasing occurrence probability of extreme events. Especially in small flash flood exposed catchments with short reaction times accurate flood forecasting is highly important. Unfortunately the available rainfall-runoff models do, especially for extreme events in small catchments, not allow for an acceptable prediction accuracy. In order to use these models for flood forecasting for emerging flood events and to ensure an acceptable accuracy the model outputs have to be continuously updated using real time river gauge measurements. Various updating procedures have been published which are, however, mostly not suitable for short forecast periods and a steep flood hydrograph characteristic which is typical for small, quick reacting mountainous catchments. In such catchments it is the primary goal to extend the forecast lead time. This requires procedures that update the state variables that govern the runoff generation process of the used rainfall runoff model. Classical updating procedures are focussed on the river flow itself which leads to a significant loss of forecast lead time in small, quick reacting catchments. Therefore, it was our intention to develop a simple updating procedure that allows for the updating of sensitive state variables that control the runoff generation approach of an appropriate rainfall runoff model. We conducted the investigations in the following steps:

- Intensive review of the state of the art literature
- Analysis of suitable rainfall runoff models with respect to the goals of the investigation and model selection.
- Conceptual set up of the updating procedure and test of the first version. Analysis of the results.
- Design of the improved version and implementation in the model.
- Test of updating procedure using data sets from different catchments.
- Assessment of results.

2. Analysis of updating procedures and models

2.1 Literature review

At the beginning of the project we conducted an intensive review of the state of the art literature with respect to updating procedures for flood forecasting (see references). The most of the published approaches aim at the updating of river discharge which is not a suitable approach for small, quick reacting catchments where the forecast lead time is short. Approaches that aim at the pre event updating of state variables were rare and it became clear that such approaches are tailored solutions that have to be seen in close context with the applied model. We considered two updating procedures, (1) Kalman Filter and (2) the approach of Yang and Michelle (2001). Measurements are assumed to be reliable and therefore no measurement update is required (one feature of the Kalman filter). The time update on the other hand, i.e. projecting forward the current state estimates, is done by our chosen model PREVAH (see below). We have found few examples of the Kalman filter application for updating procedures in literature. For a conceptual model like PREVAH, however, the filter is not the adequate choice and mathematically much too complex. In addition, PREVAH's model structure would make the implementation very difficult (see the loop structure of the model core below). Therefore we discarded the (extended) Kalman filter approach.

The approach of Yang and Michelle (2001) is based on a variation of both state and model parameters (including the reservoir constants etc.). Since we consider the watershed properties to be static and not a (random) function of the time, we discarded this approach also. Therefore, we decided to develop a simple procedure for the updating of model state variables after we selected a suitable model for our investigations.

2.2 Analysis of rainfall runoff models

At first we analysed the model WASIM-ETH (see fig. 1). The analysis led to the conclusion that this model is not suitable for the intended updating procedure. Our consideration were also confirmed at the Grenoble meeting in September 2005 by the authors of the model. The key results of the model analysis are:

- The model complexity is too high.
- It is difficult to calibrate because too many variables are involved.
- The updating of state variables leads to stability problems because of iteratively linked modules.
- The solution of the Richards equation exhibits non-convergence and stability problems when the soil moisture content was updated.
- A model run requires too much computational time.

Thus, we selected a more conceptual and simple approach. The alternative model PREVAH (Precipitation-Runoff-Evapotranspiration-Hydrotope Model, Gurtz et al. 1999) is a conceptual model based on hydrologic response units (HRU) and utilizes linear reservoirs. The most sensitive parameters of this model were identified by a sensitivity analysis:

- Soil moisture storage (SSM) and
- Upper runoff storage, (SUZ)
- Hypodermic flow

The conceptual outline of both models is displayed in Figure 1.

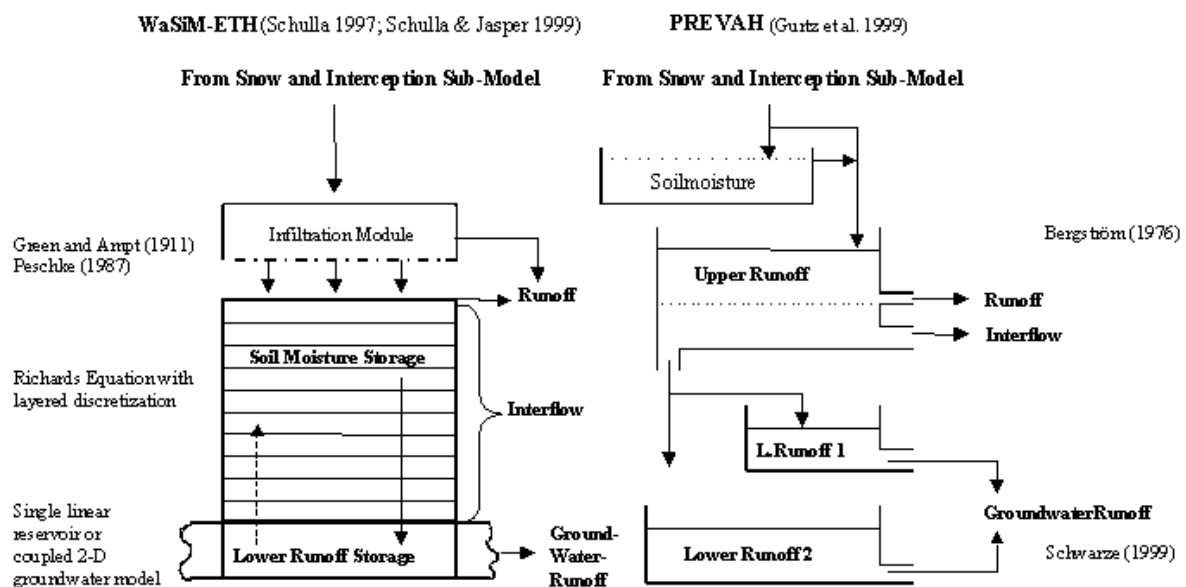


Figure 1: Conceptual outline of WASIM-ETH (Schulla, 1997; Schulla & Jaspa, 1999) and PREVAH (Gurtz et al., 1999)

3 Development and implementation of an updating procedure

3.1 Development of an updating procedure

Our proposed updating procedure is based on the simple assumption that the calculated storage of the upper soil reservoirs do not match with the real conditions. The precision criteria is the difference between the calculated and observed runoff. If it exceeds a certain threshold, the state of the upper runoff storage (and the soil moisture storage) is modified. This approach requires an iterative solution (Figure 2).

It is possible to select the hours for which updating is required. A column in the data record of the measured discharge containing an integer value controls the updating: "0" – no updating and "1" – with updating. An example for the file „q_measure.txt“ is given below:

1993	9	29	21	0.5203	0
1993	9	29	22	0.5096	0
1993	9	29	23	0.4982	0
1993	9	30	0	0.4870	1
1993	9	30	1	0.4763	1
1993	9	30	2	0.4668	1
1993	9	30	3	0.4574	1

Column 1 to 4 are reserved for the datum. Column 5 contains the observed discharge in [m³/s] and the 6th column is the updating switch. Accordingly to the value in this column, the updating is conducted never, once, at selected times, or always. It should be noted that the updating is only conducted when the difference between the simulated and observed runoff exceeds the user defined tolerance criteria ϵ . This criteria can be defined by the control file „update.inp“ which contains only three relevant lines (2-4):

- 1st Line: Dummy / standard
- 2nd Line: "Master" updating switch: [y / n]
- 3rd Line: Detailed output on the screen: [y / n]
- 4th Line: Tolerance criteria, 0.01 = 10%

We applied the secant method for the iterative determination of the factor **F**:

$$\mathbf{F}(i)^{j+1} = \mathbf{F}(i)^j - \left[Q_s^j(i) - Q_m(i) \right] \cdot \frac{\left[\mathbf{F}(i)^j - \mathbf{F}(i)^{j-1} \right]}{\left[Q_s^j(i) - Q_m(i) \right] \cdot \left[Q_s^{j-1}(i) - Q_m(i) \right]} \quad (\text{Eq.1})$$

where, $i = [1 \dots 24]$ is the count of hours during a simulation day, $j = [1 \dots 12]$ is the iteration count of the updating loop, Q_s is the simulated runoff and Q_m is the measured runoff. Convergence is usually achieved within 3 or 4 iterations.

For certain (theoretical) model states, the updating of SUZ and SSM will not lead to a minimization of the objective function to the required precision criteria. If simulated runoff is over-predicted and SUZ/SSM already empty for example, it can not be further reduced. In such a case the best solution is kept and the iteration proceeds to the next time step. As a side effect, this approach also damps the effect of measurement errors (extreme values).

4. Test and application

At first we tested the updating procedure on the well observed Verzasca catchment - a mountainous catchment in Switzerland with an area of 186 km². Figure 3 shows the observed discharge, the full calibrated simulation results of PREVAH and a run with updating of the same event. Until the simulation time 2/10/1993, 7:00 a.m., the updating procedure is applied. From this simulation time onwards, PREVAH predicts the discharge (without updating). The prediction of the peak discharge at 2:00 p.m is significantly better compared to the model result of the calibration run.

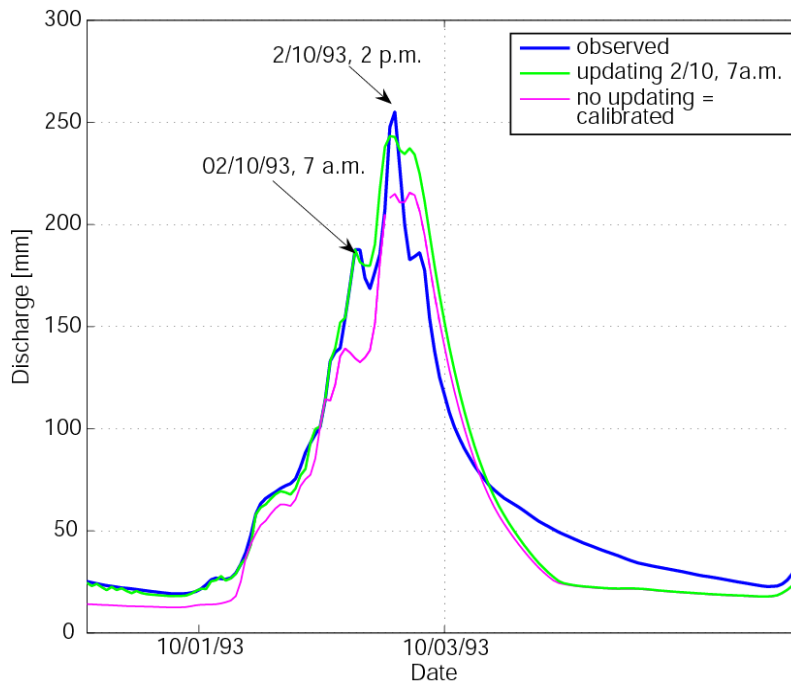


Figure 3: Comparison of measured and simulated discharge (Verzasca catchment): 1) the calibrated run and 2) a run with updating and projection

For test purposes, the above flood event was simulated assuming runoff measurements available for the entire event. Updating the model state hourly yields a perfect match of the calculated runoff and the measurements (Figure 4). The volume added/subtracted to the storage (SUZ, SSM) as a result of the updating is shown in Figure 4. If the model state is continuously (hourly) updated, the balance

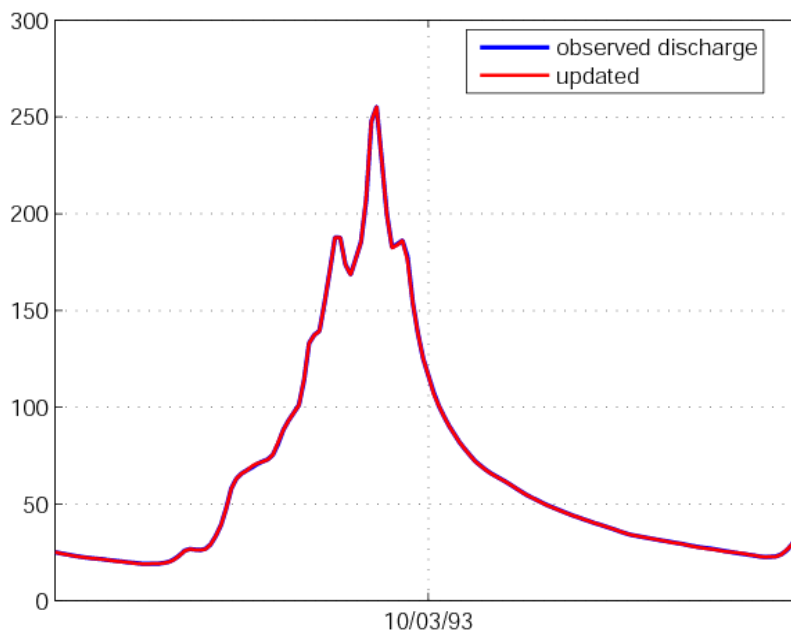


Figure 4: Comparison of the observed and simulated discharge (Verzasca catchment) with full (continuous) updating

of added/removed water volume during the entire event is +26.3mm. It should be noted that it is much less, if the model state is only updated at specific times of the rising discharge hydrograph (see above) and the flood peak prediction still can be significantly improved. Usually, the tailing (falling) discharge hydrograph is of lesser importance in flood forecasting.

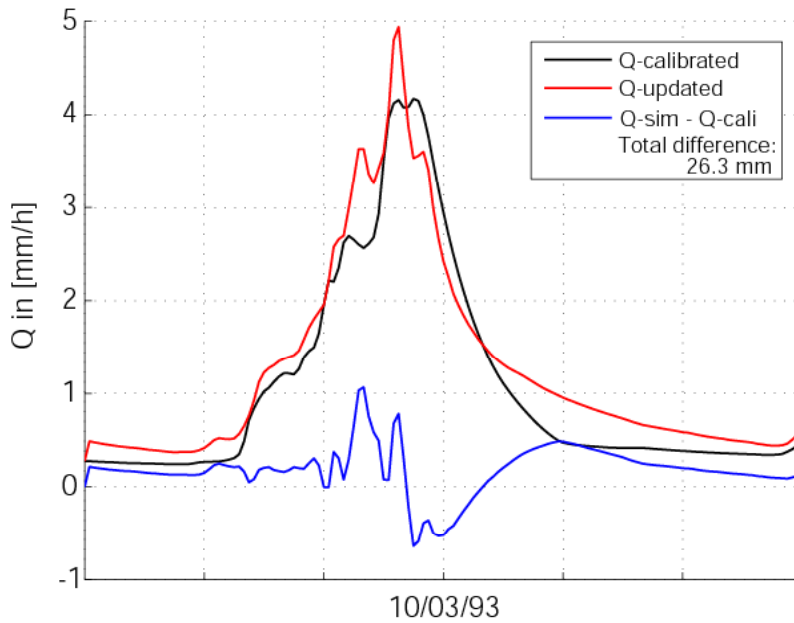


Figure 5: Measured discharge, calibrated PREVAH simulation, updated volume during flood event

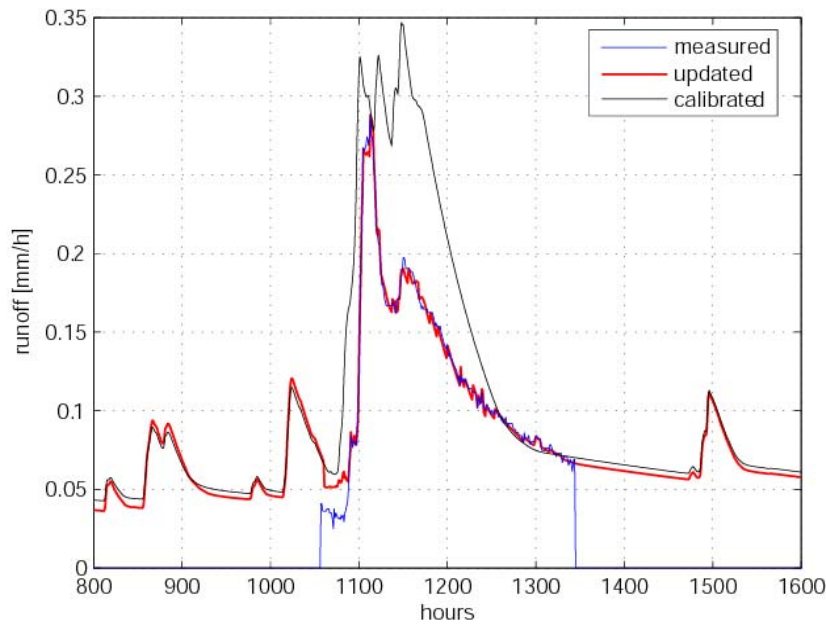


Figure 6: River Mulde catchment, gauge Wechselburg: Comparison of the calibrated PREVAH run, the updated PREVAH run and the measurements of a single flood event

The developed updating procedure was meanwhile implemented in the PREVAH model and successfully tested on some additional examples by M. Zappa (WSL, Switzerland).

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